

ISL Sewer Modeling Report



Date November 06, 2012

Our Reference: 30387

Village of Pemberton

PO Box 100
7400 Prospect Street
Pemberton, BC V0N 2L0

Attention: Caroline Lamont, Manager of Development Services

Dear Madam:

Reference: **Village of Pemberton Sanitary Sewer Forcemain Analysis**

1.0 Introduction

As requested, an analysis has been conducted on the Village of Pemberton's sanitary forcemain system and wastewater treatment plant. The purpose of the analysis was to determine if there is capacity in the existing sanitary forcemain and treatment plant to accept the proposed sanitary sewer flows anticipated from the proposed development. In particular, an analysis was conducted for the following scenarios

- The existing flow conditions of the Village of Pemberton forcemain system,
- The existing flow conditions plus the Sunstone Ridge Development Phase 1, and
- The existing flow conditions plus the Sunstone Ridge Development Phase 1 & 2 plus future developments in the area.

2.0 Design Criteria

The design criteria for the analysis were taken from the Village of Pemberton Subdivision and Control Bylaw 677 and the Master Municipal Construction Document (MMCD).

3.0 The Existing Sanitary System

The Village's sanitary system, as it relates to this analysis, consists of a sewage pump station located at industrial park and a forcemain running from the pump station to the treatment plant. The forcemain generally runs west from the industrial park along Highway #99 until a bend at Sturdy's Farm approximately 500m west of the intersection of Industrial Park and Highway #99. According to record drawings, there is a 200x200x200 HDPE Tee with 200mm blind flange immediately upstream of the bend at Sturdy's farm. From Study's Farm, the forcemain runs south to cross under the Lillooet River and into the Village treatment plant adjacent to Airport Road.



According to record information, the existing forcemain pipe characteristics are shown in the table below:

Pipe Section	Diameter, Type, Class	Length (m)	Pressure Rating (psi)
Industrial Park to River	200mm HDPE DR26	1120	64
River Crossing	200mm HDPE DR11	220	160
River to WWTP	200mm HDPE DR26	140	64

According to record information, the pump motors at the Industrial Park pump station is currently operating with Myers pumps identified as:

Manufacturer Myers
 Model 4RCX
 Type 20Hp, 3450 RPM, 3 phase 208 volts
 Capacity 16 L/s @ 30m head

3.1 Capacity Review of Forcemain

The sanitary flows from the Industrial Park are 26 L/s including existing and future long term build-out capacities. An analysis was conducted on the existing forcemain using the 26 L/sec flow rate with the following summary of results;

Pipe Pressure Rating	64 psi
Normal Operating Pressure:	23 psi
Available capacity	64%
Short Term Pipe Rating (during surge occurrences)	96 psi
Surge Pressure:	39 psi
Total Pressure (operating + surge)	62 psi
Available capacity	35%

Based on the above analysis, the existing forcemain is sufficient for current and future flows anticipated from the Industrial Park pump station. The existing pumps will, however, need to be replaced or modified to meet long term build-out requirements.

3.2 Capacity Review of WWTP

The Village of Pemberton wastewater treatment plant, commissioned in 2005, was originally designed for a population of 5,000 people with the following design criteria:

- Average Dry Weather Flow (ADWF) 1,530 m³/d
- Maximum Daily Flow (MDF) 3,060 m³/d (or, 2*ADWF)
- Peak Wet Weather Flow (PWWF) 53 L/s

The Village maintains daily records of the flows received by the WWTP. Records indicate daily flows of up to 2,400 m³/d.



Capacity constraints of the treatment plant needs to be reviewed further in order to assess the effects of the proposed development on the existing system. High inflow and infiltration rates may have significant impacts to the available capacity at the existing treatment plant.

4.0 The Existing Sanitary System with Future Developments

4.1 Sunstone Ridge Development Phase 1

Phase 1 of the Sunstone Ridge Development (SRD) is located approximately 3 km east of the Village core, north of the CN railway. The SRD site will be the first of a number of developments in the Sunstone Ridge area and will consist of single family and multi-family units. The total demands identified for SRD were taken from the Delcan Technical Memorandum dated April 16, 2012 and are as follows:

- ADWF = 3.5 L/s
- I&I = 3.9 L/s
- PWWF = 15.3 L/s

The full Technical Memorandum can be found in Appendix B of this report.

With the development of the SRD site, a new pump station and forcemain is proposed. The forcemain tie-in is proposed at the existing forcemain on Highway #99 near Sturdy's Farm.

The analysis of the existing forcemain capacity as it relates to Phase 1 proposed development with existing flows from Industrial Park is summarized below:

Pipe Pressure Rating	64 psi
Normal Operating Pressure:	28 psi
Available capacity	56%
Short Term Pipe Rating (during surge occurrences)	96 psi
Surge Pressure:	61 psi
Total Pressure (operating + surge):	89 psi
Available capacity	7%

4.2 Sunstone Ridge Development Phase 1 & 2 plus Future Developments

Future development areas in addition to the SRD Phase 1 are outlined in Table 1 of Delcan's Technical Memorandum and include a school site, recreational facility, Biro site, commercial site, 22 SF site, and SRD Phase 2. The total demands from the Technical Memorandum are summarized in the following table.

Site	ADWF (L/s)	I&I (L/s)	PWWF (L/s)
SRD Site (Phase 1)	3.5	3.9	15.3
School Site	4.0	1.1	14.0
Recreation Facility	1.6	1.5	7.2
Biro Site	2.7	3.4	14.4
Commercial Site	0.1	0.1	0.5
22 SF Units	0.4	0.3	2.0
SRD Site (Phase 2)	2.5	5.6	13.9

The analysis of the existing forcemain capacity as it relates to Phase 1 & 2 and future developments with existing flows from Industrial Park is summarized below:

Pipe Pressure Rating	64 psi
Normal Operating Pressure:	64 psi
Available capacity	0%
Short Term Pipe Rating (during surge occurrences)	96 psi
Surge Pressure:	127 psi
Total Pressure (operating + surge)	191 psi
Available capacity (<i>capacity is exceeded</i>)	0%

5.0 Conclusions

The following conclusions are made based on the above:

- Phase 1 of the Sunstone Ridge Development may be accommodated with the existing forcemain.
- Only the portion of existing forcemain under the Lillooet River (160 psi pipe rating) can accommodate Phase 1 & 2 and future developments.
- The portion of the existing forcemain (64 psi pipe rating) cannot accommodate all flows anticipated from Phase 1 & 2 and future development.
- An evaluation of the treatment plant capacity needs to be completed to assess the effects of the proposed development. This may include completion of an inflow and infiltration study.

6.0 Recommendations

1. Although the first phase of SRD may be accommodated with the existing forcemain, it is recommended that that any proposed connection to the existing forcemain be made downstream of the transition between the existing DR 26 and DR 11 forcemain pipe. In addition, the forcemain section on the south of the Lillooet River requires upgrading or a second parallel pipe be installed to the treatment plant to accommodate build-out plans beyond Phase 1 of the Sunstone Ridge Development.

2. An evaluation of the treatment plant capacity is recommended to determine the affects of future development impacts.
3. An inflow and infiltration study may be required to determine the contribution of stormwater or groundwater to the sanitary sewer flows.

7.0 Closure

This report is prepared in response to the Draft Technical Memorandum prepared by Delcan dated April 16, 2012. Please contact the undersigned if you have any questions.

Prepared by,



Richard Avedon-Savage, P.Eng.
Project Engineer

Reviewed by,



Graham Schulz, P.Eng.
Senior Project Engineer

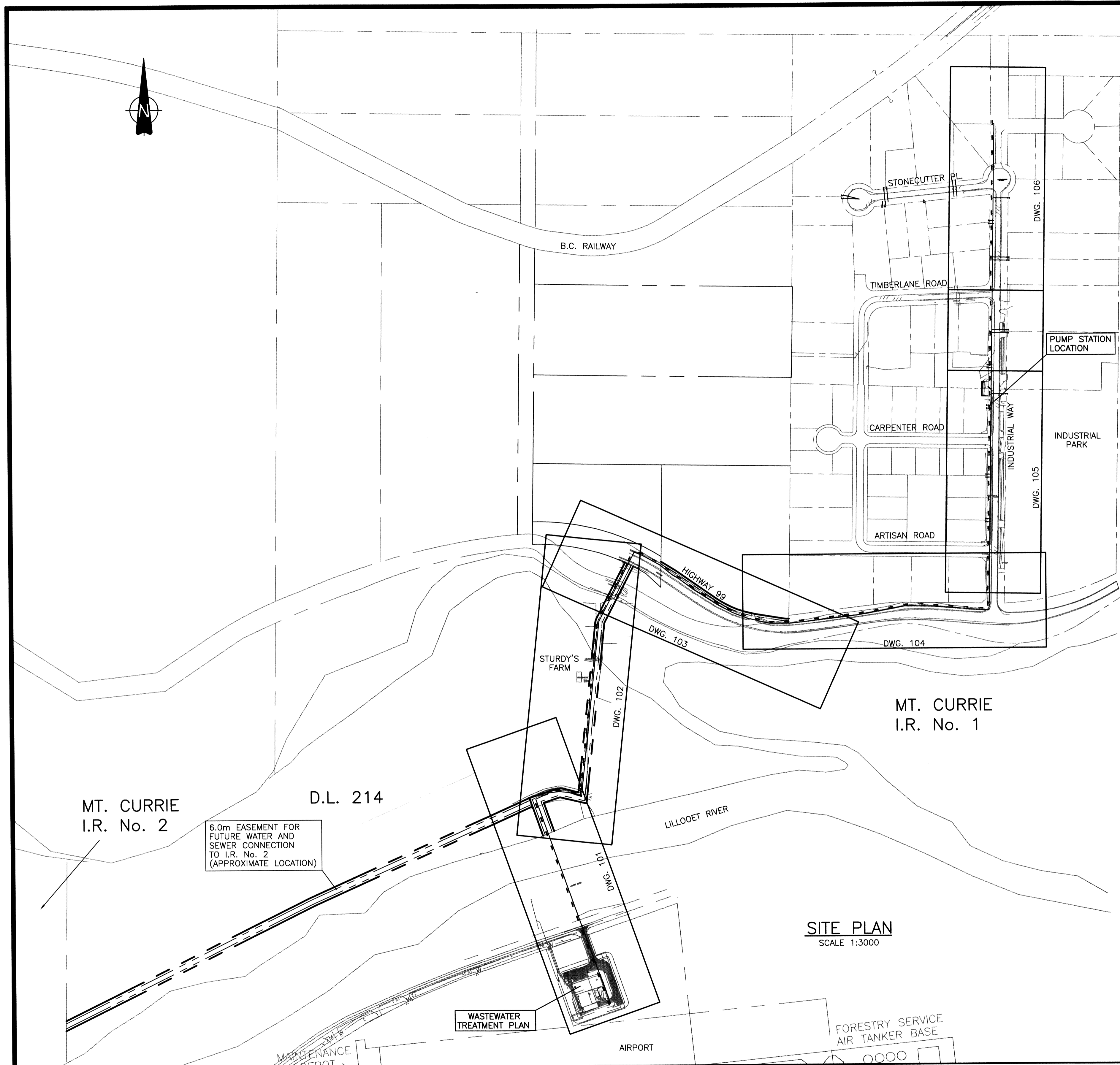
Attachments



Appendix A – Record Drawings



This Drawing is For The Use Of The Client And Project Indicated
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I.R. No. 10



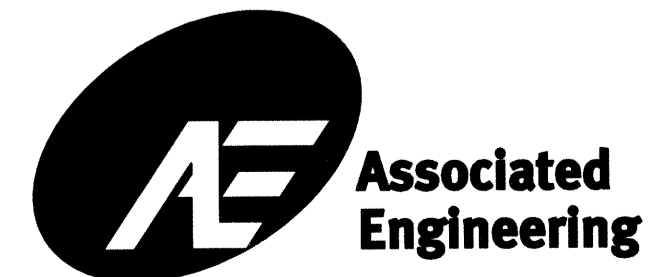
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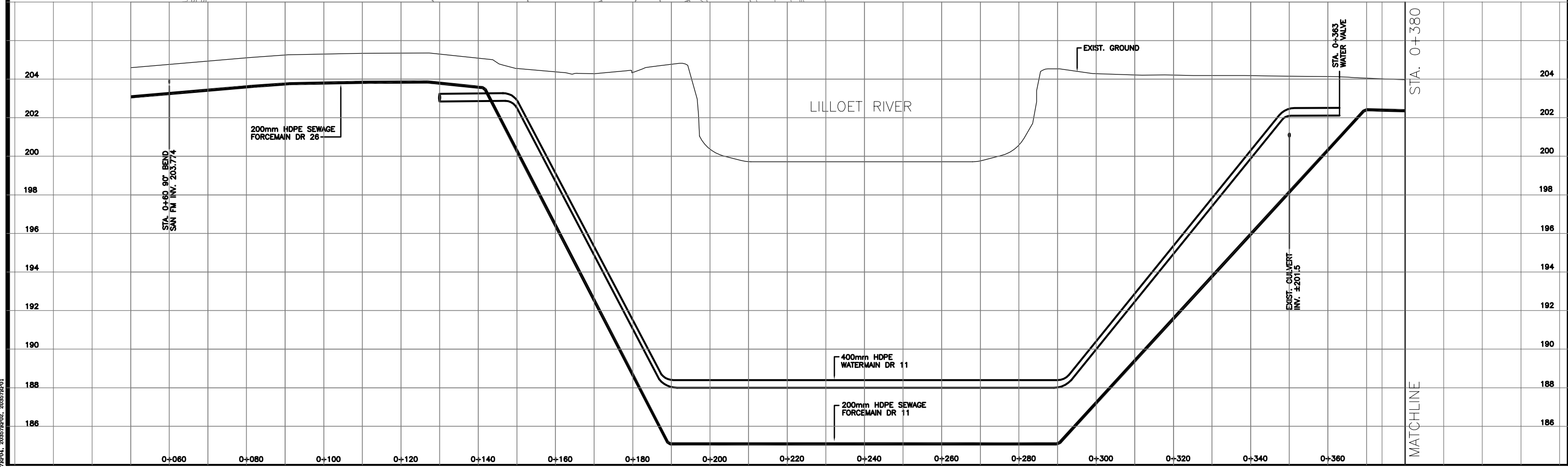
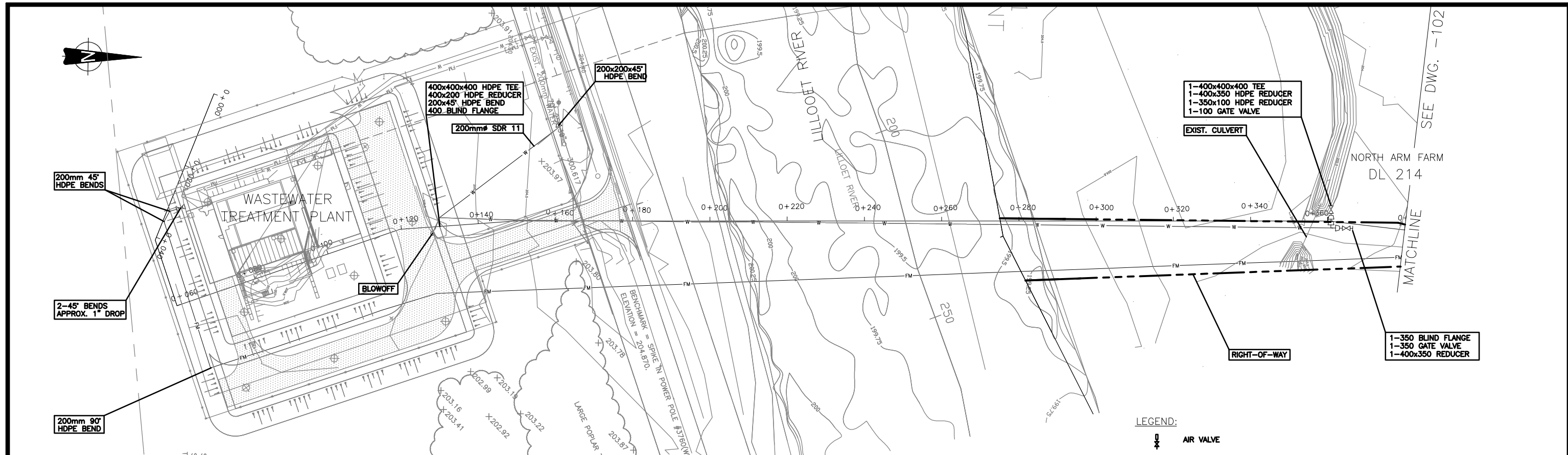


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CHECKED	D.D.		05/07/25
APPROVED	E.A.B.		05/07/25
DATE		INITIAL	

VILLAGE OF PEMBERTON
LOCATION PLAN AND SITE PLAN

INDUSTRIAL PARK OFF-SITE WORKS		
DRAWING NUMBER	REV. NO.	SHEET
002035-7-100		

This Drawing is For The Use Of The Client And Project Indicated
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Drawing: 002035-7-101
Author: J. Smith
Check: J. Smith
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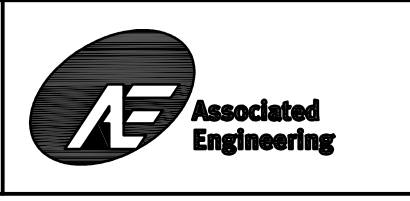
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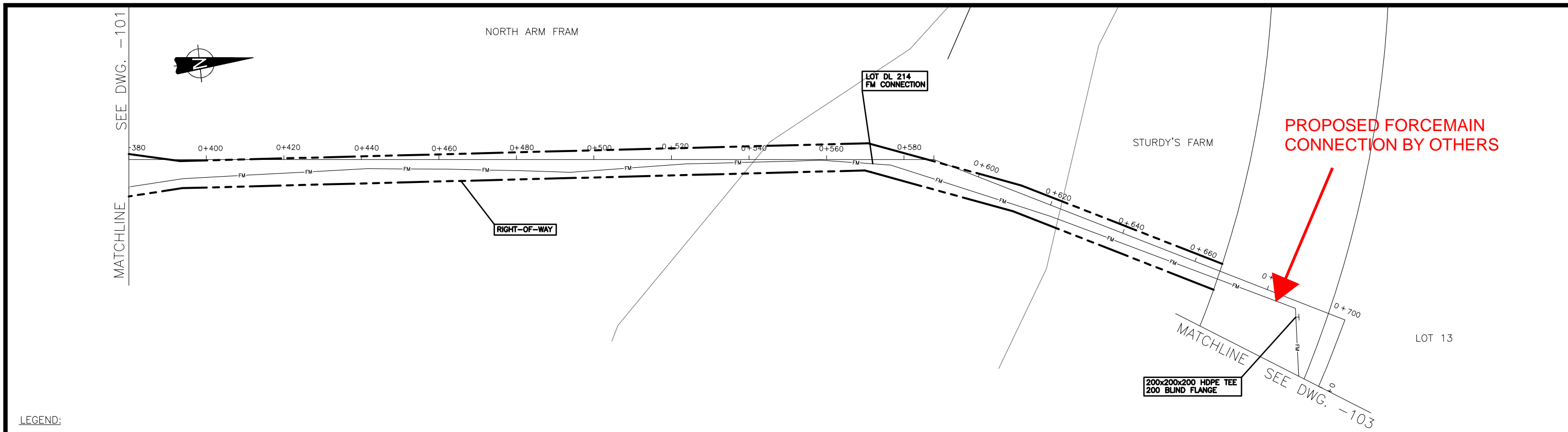
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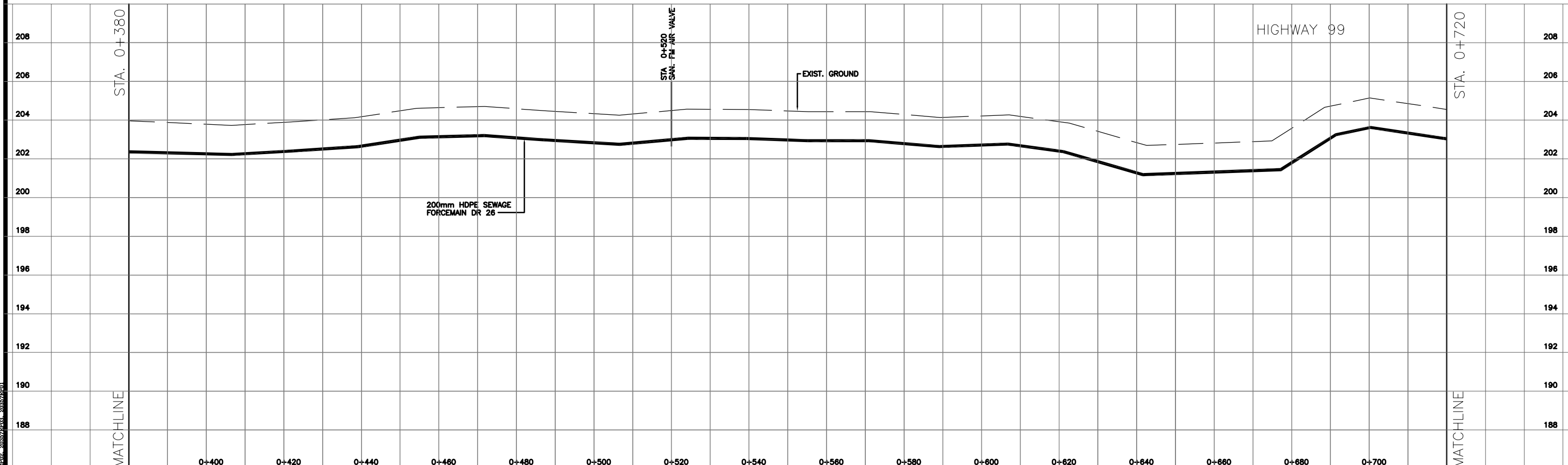
SANITARY FORCEMAIN
PLAN AND PROFILE - SHEET 1

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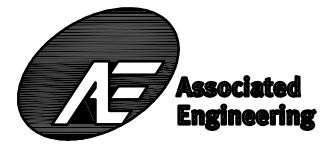
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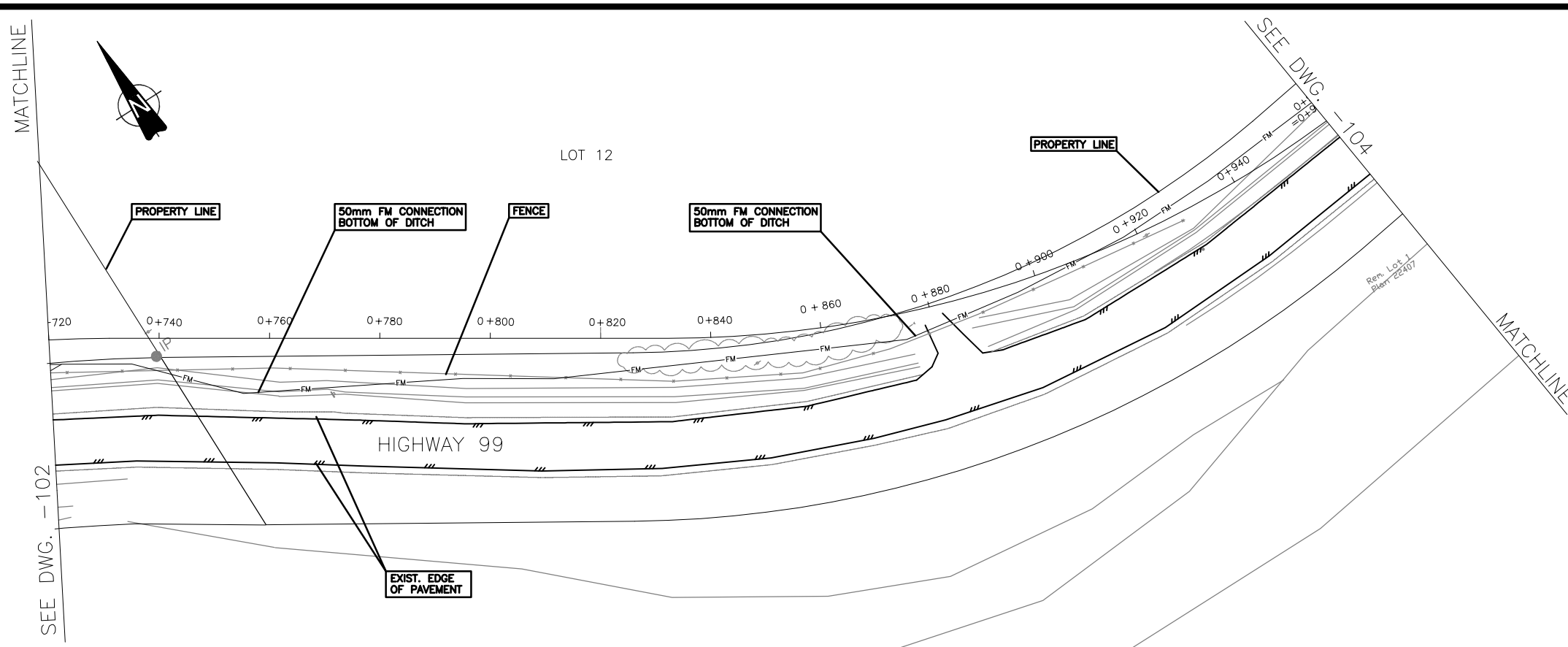


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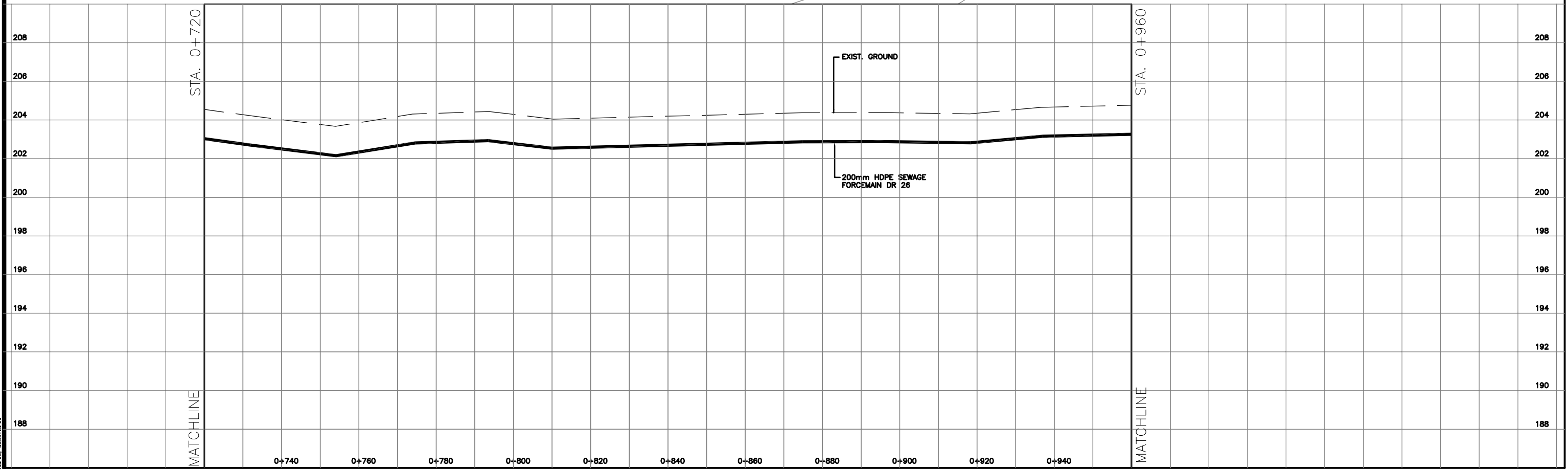
VILLAGE OF PEMBERTON
 SANITARY FORCEMAIN
 PLAN AND PROFILE - SHEET 2

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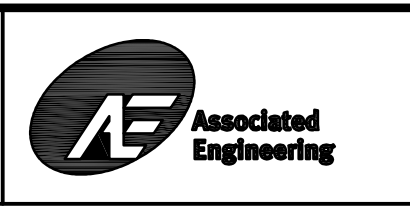
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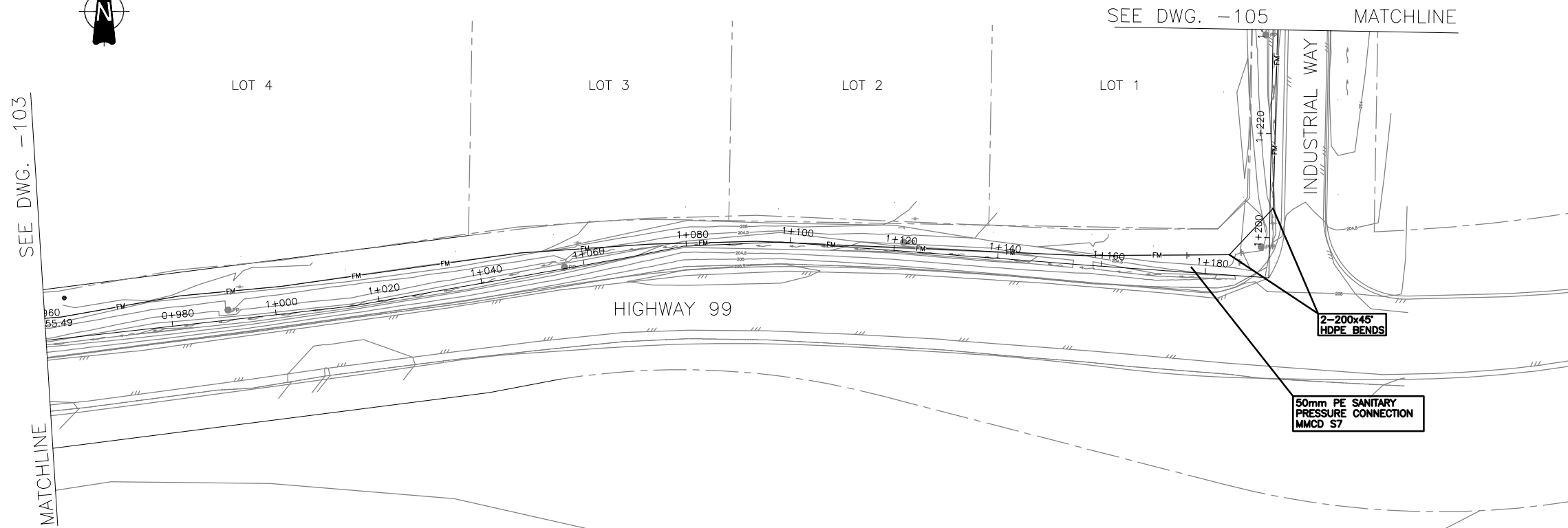


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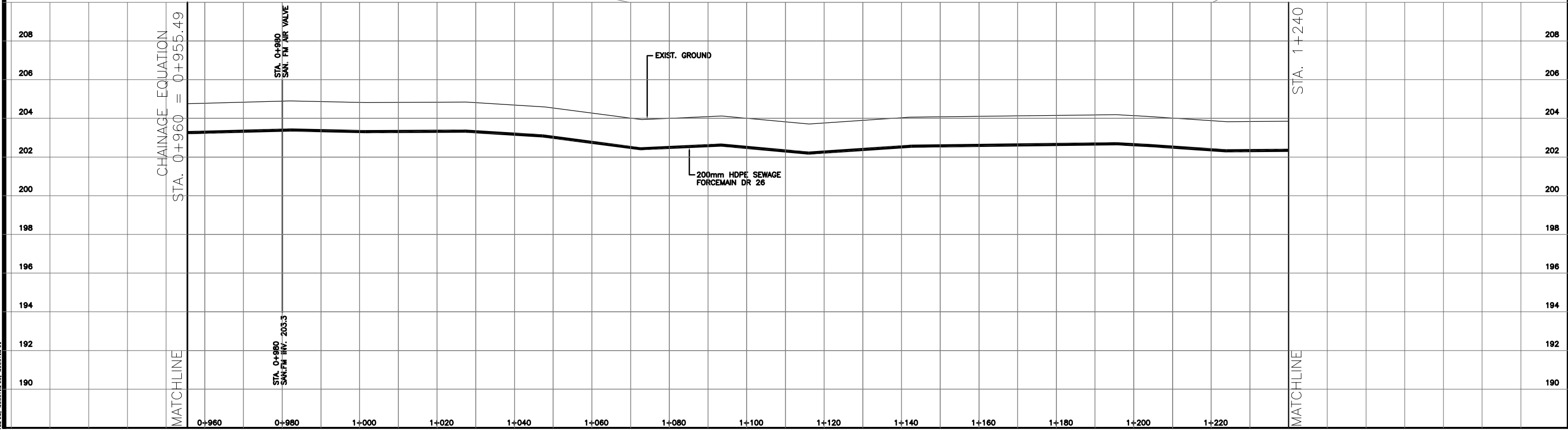
VILLAGE OF PEMBERTON
 SANITARY FORCEMAIN
 PLAN AND PROFILE - SHEET 3

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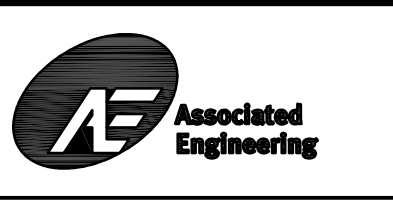
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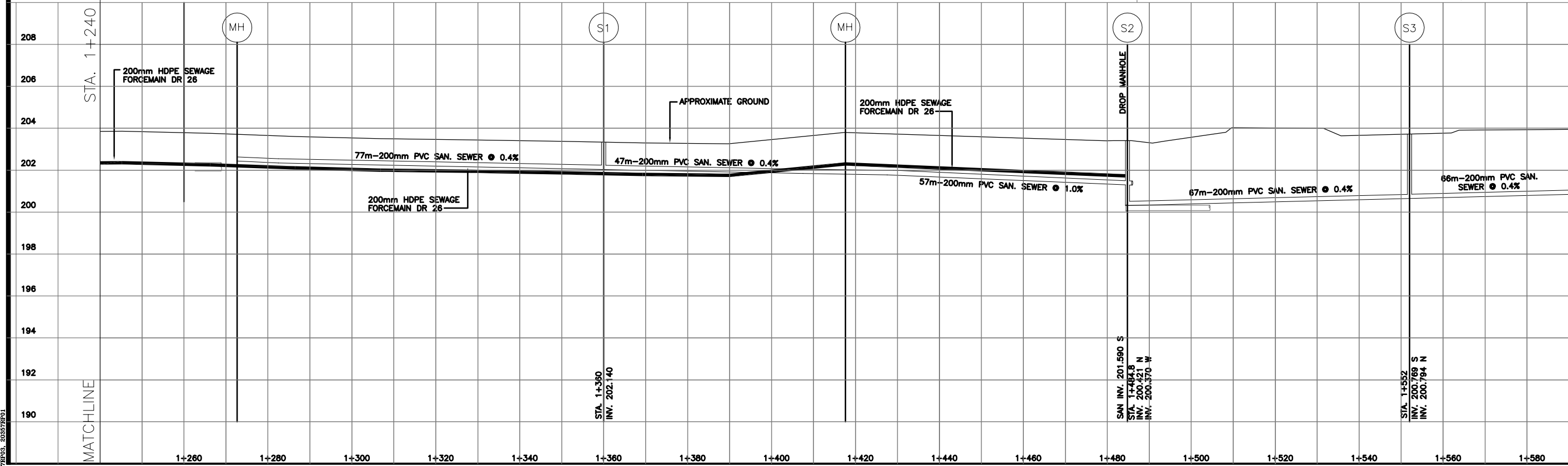
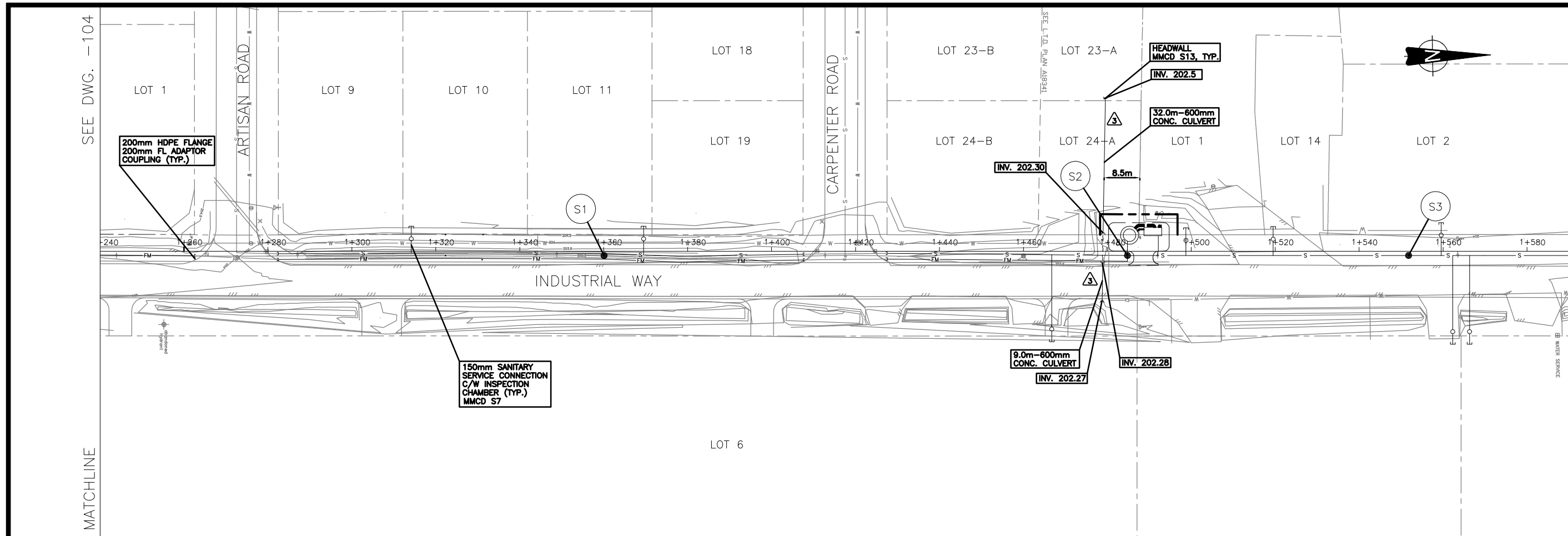


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APPROVED	E.A.B.		05/07/26
DATE		INITIAL	

VILLAGE OF PEMBERTON
 SANITARY FORCEMAIN
 PLAN AND PROFILE - SHEET 4

INDUSTRIAL PARK OFF-SITE WORKS		
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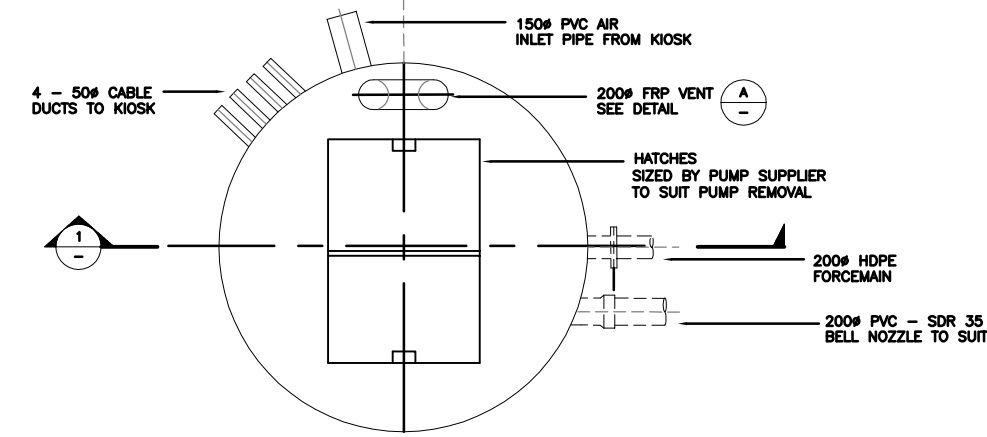
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VILLAGE OF PEMBERTON

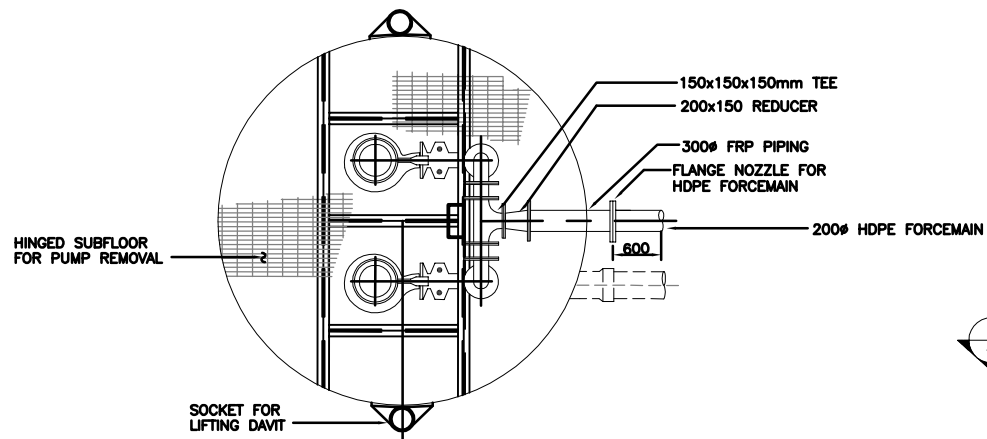
SANITARY FORCEMAIN
PLAN AND PROFILE - SHEET 5

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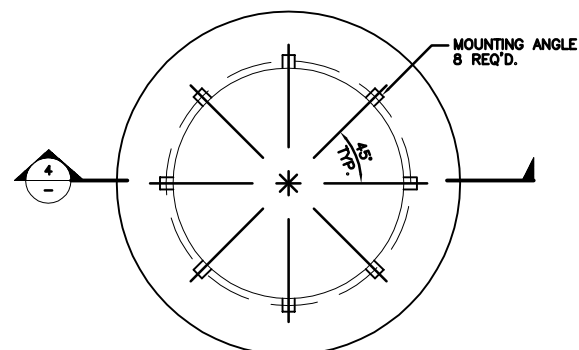
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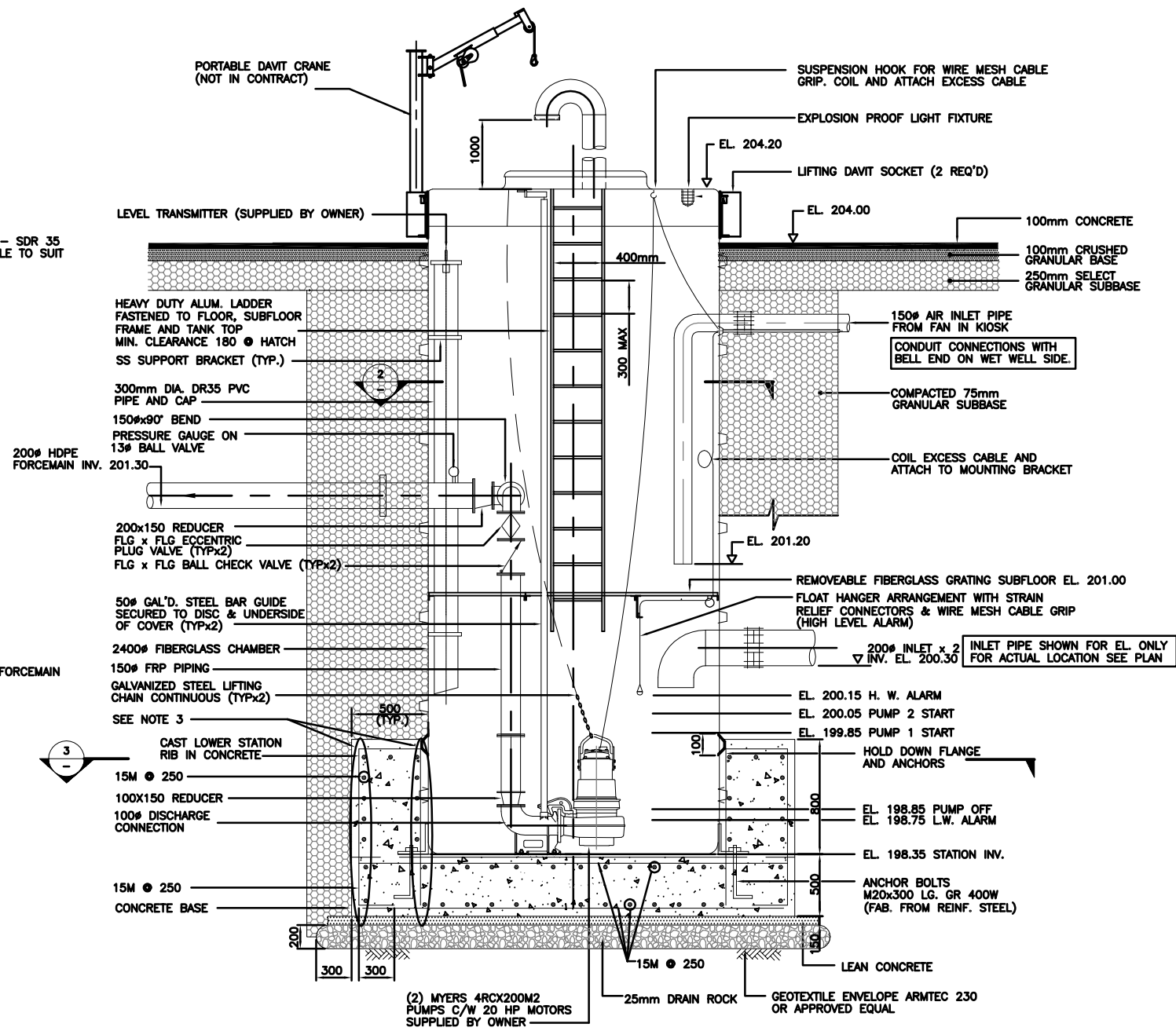
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SECTION 2
NTS

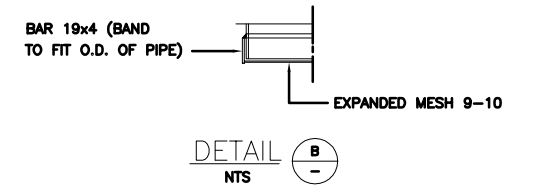
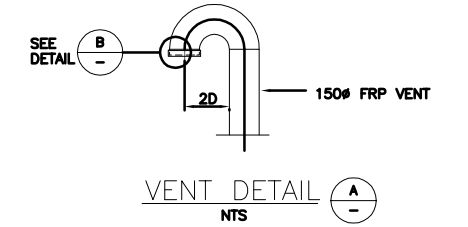


SECTION 3
NTS



SECTION 1
NTS

- NOTES:
1. SEE PLAN FOR TRUE ORIENTATION
2. FIELD ROUTE TO KIOSK, MIN. BEND RADIUS 200mm
3. BARS BENT TO FORM CIRCLE



Date: 2017/7/28
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NO.	DATE	ENG.	BY	SUBJECT
4	07/07/26	D.D.	L.Y.	RECORD DRAWING
3	06/09/15	E.A.B.	C.L.	PUMPS AND LEVEL TRANSMITTER
2	06/08/24	D.D.	C.L.	ISSUED FOR CONSTRUCTION
1	06/04/07	E.A.B.	C.L.	ISSUED FOR TENDER
REVISIONS				

VERIFY SCALES	
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IF NOT 20mm ON THIS SHEET, ADJUST SCALES ACCORDINGLY	

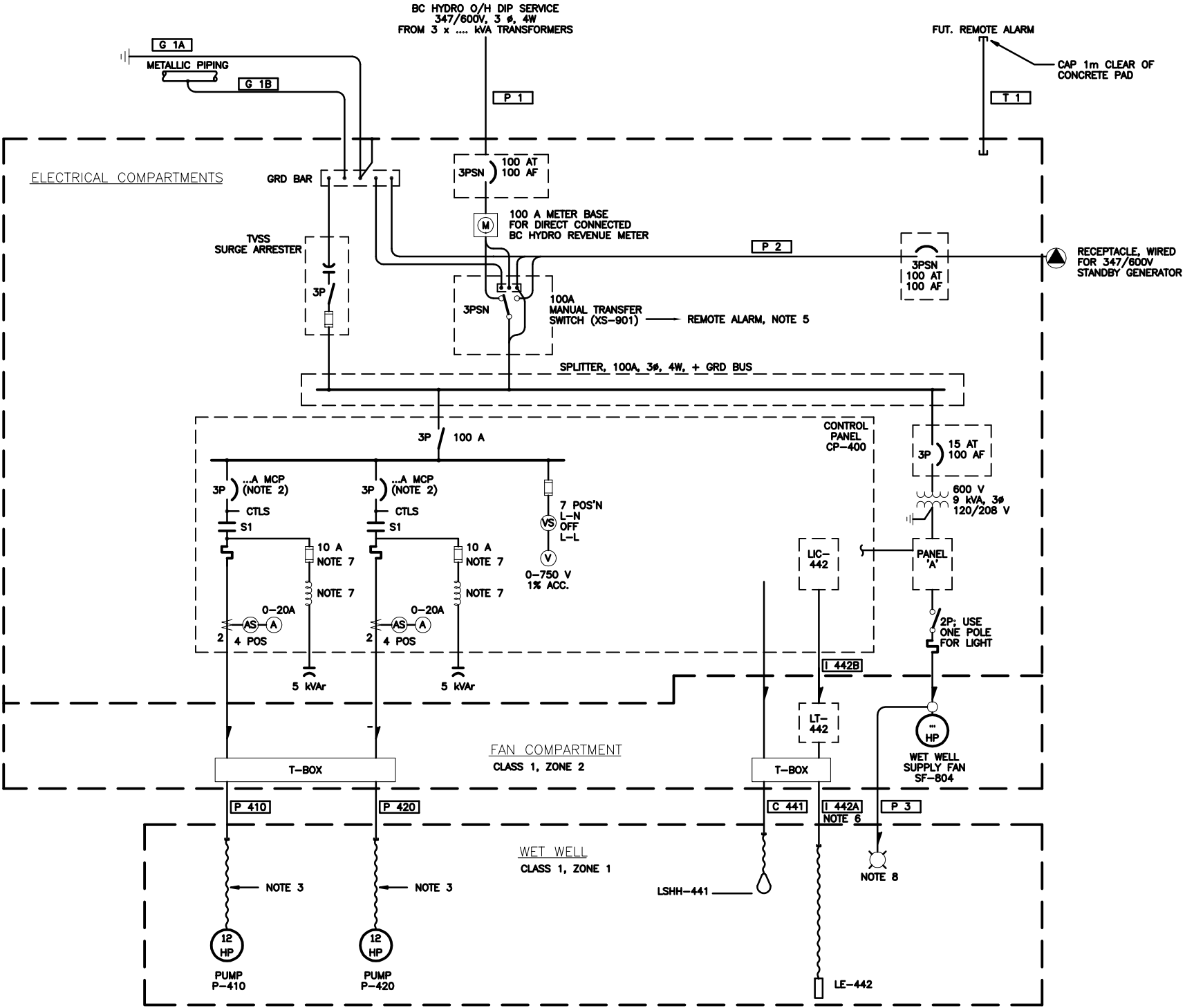


PROJECT No.	002035
SCALE	AS NOTED
DRAWN	C.L. 05/09/09
DESIGNED	E.A.B. 05/09/09
CHECKED	E.A.B. 05/09/09
APPROVED	E.A.B. 05/05/09
DATE	INITIAL

VILLAGE OF PEMBERTON	
INDUSTRIAL PARK OFF-SITE SERVICES	
SANITARY SEWER LIFT STATION	

DRAWING NUMBER	REV. NO.	SHEET
002035-7-402	4	16/18

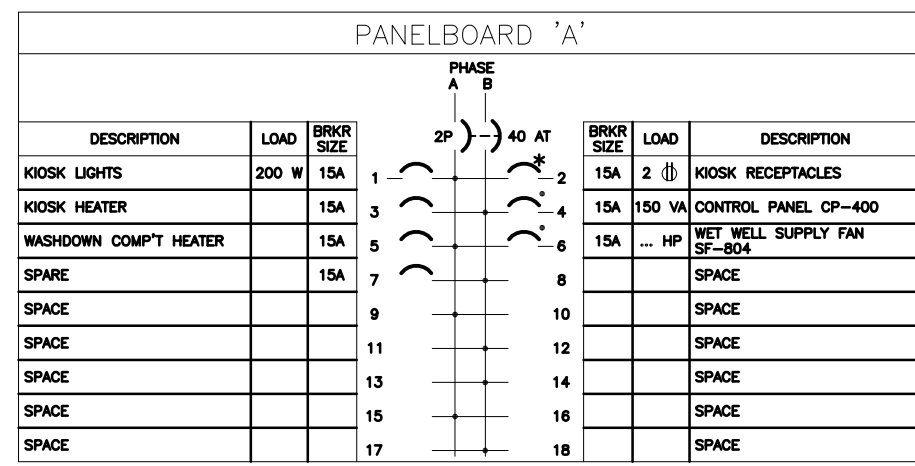
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SINGLE LINE DIAGRAM

WIRING SCHEDULE (NOTE 4)		
P 1	78 DB2 C.O.	BC HYDRO SERVICE CONDUIT
P 2	35 RPVC, 3 #3 + #6 GRD	STANDBY GENERATOR RECEPTACLE
P 3	41 RPVC, 2 #12 + #14 GRD	WET WELL LIGHT
*P 410	53 RPVC, 3 PWR + GRD COND'S + 4 SENSOR LEADS FOR PUMP	PUMP P-410 POWER + SENSOR LEADS FLEXIBLE CABLE(S)
*P 420	53 RPVC, 3 PWR + GRD COND'S + 4 SENSOR LEADS FOR PUMP	PUMP P-420 POWER + SENSOR LEADS FLEXIBLE CABLE(S)
*C 441	41 RPVC, FLEXIBLE CABLE SUPPLIED WITH FLOATSWITCH	FLOAT-TYPE LEVEL SWITCH IN WET WELL
*I 442A	53 RPVC, SHIELDED CABLE / TUBE FACTORY-SUPPLIED	LEVEL TRANSDUCER IN WET WELL (NOTE 6)
I 442B	21 RA, 1 TP5H + #14 GRD	LEVEL TRANSMITTER
T 1	53 DB2 C.O.	FOR FUTURE TELUS SERVICE
G 1A	21 RPVC, #6 GRD	
G 1B	21 RPVC, #6 GRD	

* INSIDE WET WELL AND FAN COMPARTMENT CHANGE TO RA



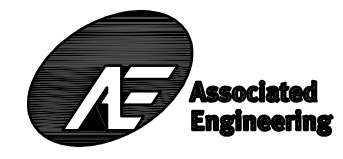
* DENOTES GFCI BREAKER • WITH PAD LOCK DEVICE IN ON / OFF POSITION

- NOTES:
- SERVICE SIZED FOR FULL PUMPING CAPACITY, PLUS ANCILLARY LOADS.
 - MCP SIZE AND TRIP RANGE TO BE SELECTED BY MANUFACTURER BASED ON MOTOR FLC; TRIP SETTING OF MCP AND OVERLOADS TO BE ADJUSTED ACCORDINGLY.
 - COMPOSITE CABLE OR SEPARATE POWER AND MONITOR CABLES, SUPPLIED WITH PUMP; CONFIRM WITH PUMP SUPPLIER.
 - SIZE INTERNAL KIOSK WIRING IN ACCORDANCE WITH THE RULES AND REGULATIONS.
 - PROVIDE ISOLATED 120 VAC CONTACTS, WIRED TO TERMINAL BLOCKS, FOR THE FOLLOWING FUTURE REMOTE ALARMS:
 - PUMP P-410 RUN (MS-410)
 - PUMP P-420 RUN (MS-420)
 - PUMP P-410 FAULT (ISH-410, TSH-410, ES-410)
 - PUMP P-420 FAULT (ISH-420, TSH-420, ES-420)
 - TRANSFER SWITCH IN EMERGENCY POSITION (XS-901)
 - INTRUSION
 - WET WELL HIGH LEVEL (LAHH-441B)
 - WET WELL LOW LEVEL (LAL-442)
 - MINIMUM CONDUIT RADIUS REQUIRED FOR TRANSDUCER CABLE IS 200 mm.
 - DUAL-ELEMENT TYPE FUSE; PROVIDE 10 COILED TURNS IN THE CAPACITOR LEADS.
 - COMPACT FLUORESCENT LUMINAIRE, CLASS 1, ZONE 1, WITH 26W LAMP AND GUARD, NRL #ZM-F-26-G--C

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2	06/08/24	D.D.	C.L.	ISSUED FOR CONSTRUCTION
1	06/04/07	E.A.B.	C.L.	ISSUED FOR TENDER

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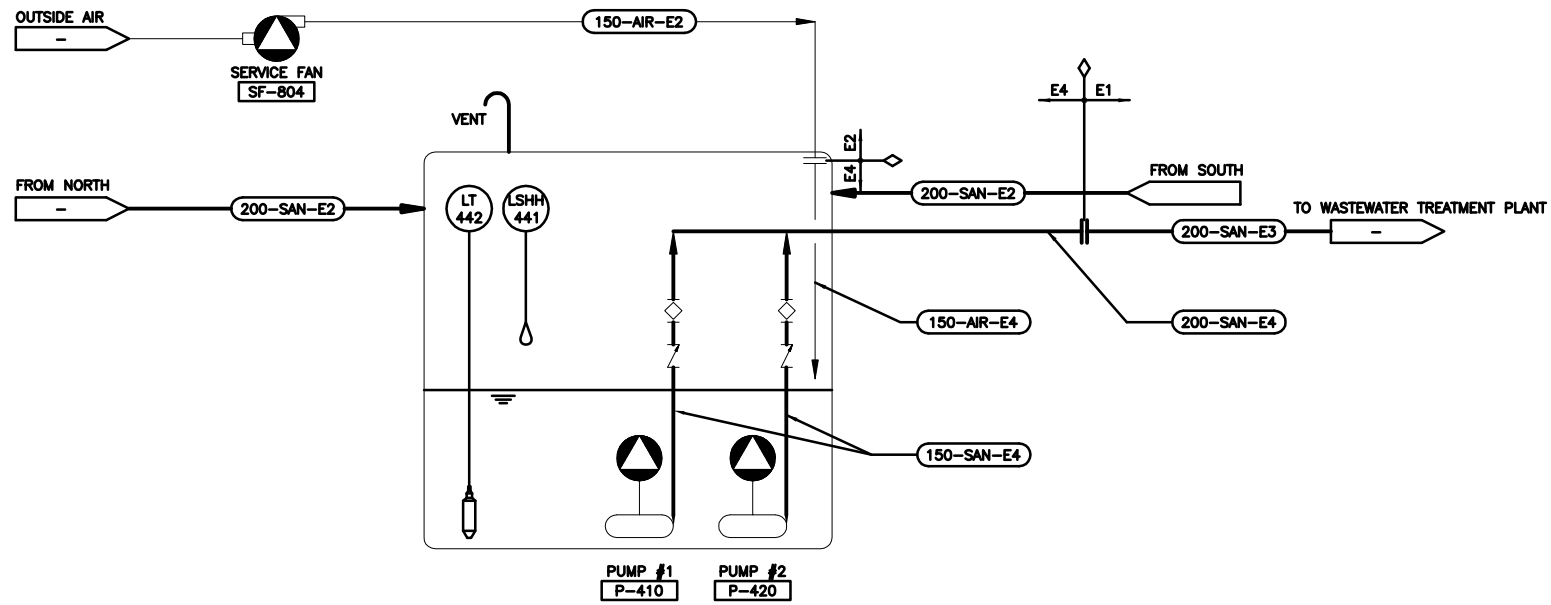


PROJECT No.	002035		
SCALE	NONE		
DRAWN	J. THEILER		05/08/22
DESIGNED	J. THEILER		05/08/22
CHECKED	E.A.B.		05/08/22
APPROVED	E.A.B.		05/08/22
DATE		INITIAL	

VILLAGE OF PEMBERTON
 INDUSTRIAL PARK OFF-SITE SERVICES
 ELECTRICAL PUMP STATION
 SINGLE LINE DIAGRAM

DRAWING NUMBER	REV. NO.	SHEET
002035-7-601	3	17/18

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INDUSTRIAL PARK PUMP STATION

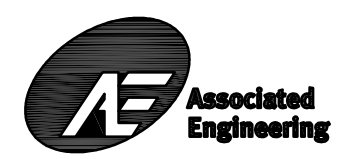
LEGEND

	FAN		BALL VALVE		LINE CODE
	SUBMERSIBLE PUMP		CHECK VALVE		MATERIAL
	SUBMERSIBLE MIXER		LEVEL SWITCH (LS)		A1- STEEL PIPE, SCH 40, COAL TAR EPOXY LINED AND COATED
	PLUG VALVE		LEVEL TRANSMITTER (LT)		E1- PVC PIPE SCHED 80
	GATE VALVE		BACKFLOW PREVENTER		E2- PVC PIPE
			PRESSURE CONTROL VALVE (DOWN STREAM SENSING)		E3- POLYETHYLENE PIPE
			BLIND FLANGE		H1- STAINLESS STEEL, SCH 40, SOCKET WELD
					COMMODITY
					AIR-SERVICE AIR
					PW-POTABLE WATER
					SAN-SANITARY WASTE
					LINE SIZE (IN MILLIMETRES)

Time: 8:36 Date: 20/07/28 Puser Specs: AutoCAD File: G:\060335\2005 INDUSTRIAL PARK OFF SITE SERVICES\CIVIL\SUBMISSION\AS BUILTS_JULY_28\20057401.DWG (L.Y.)

NO.	DATE	ENG.	BY	SUBJECT
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2	06/08/24	D.D.	C.L.	ISSUED FOR CONSTRUCTION
1	06/04/07	E.A.B.	C.L.	ISSUED FOR TENDER
REVISIONS				

VERIFY SCALES
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PROJECT No.	002035
SCALE	AS NOTED
DRAWN	C.L. 05/09/09
DESIGNED	E.A.B. 05/09/09
CHECKED	E.A.B. 05/09/09
APPROVED	E.A.B. 05/09/09
DATE	INITIAL

VILLAGE OF PEMBERTON
PROCESS MECHANICAL
P & ID, LEGEND

INDUSTRIAL PARK OFF-SITE WORKS		
DRAWING NUMBER	REV. NO.	SHEET
002035-7-401	3	15/18

Appendix B – Delcan Technical Memorandum



DRAFT Technical Memorandum

To: Cam McIvor, Project Manager
cc: Grant Campbell, P.Eng

Date: April 16, 2012

From: Colin Kristiansen, P.Eng
Todd Bowie, P.Eng

Our Ref: EB3766

RE: Sunstone Ridge Development – Sanitary Loading Assessment

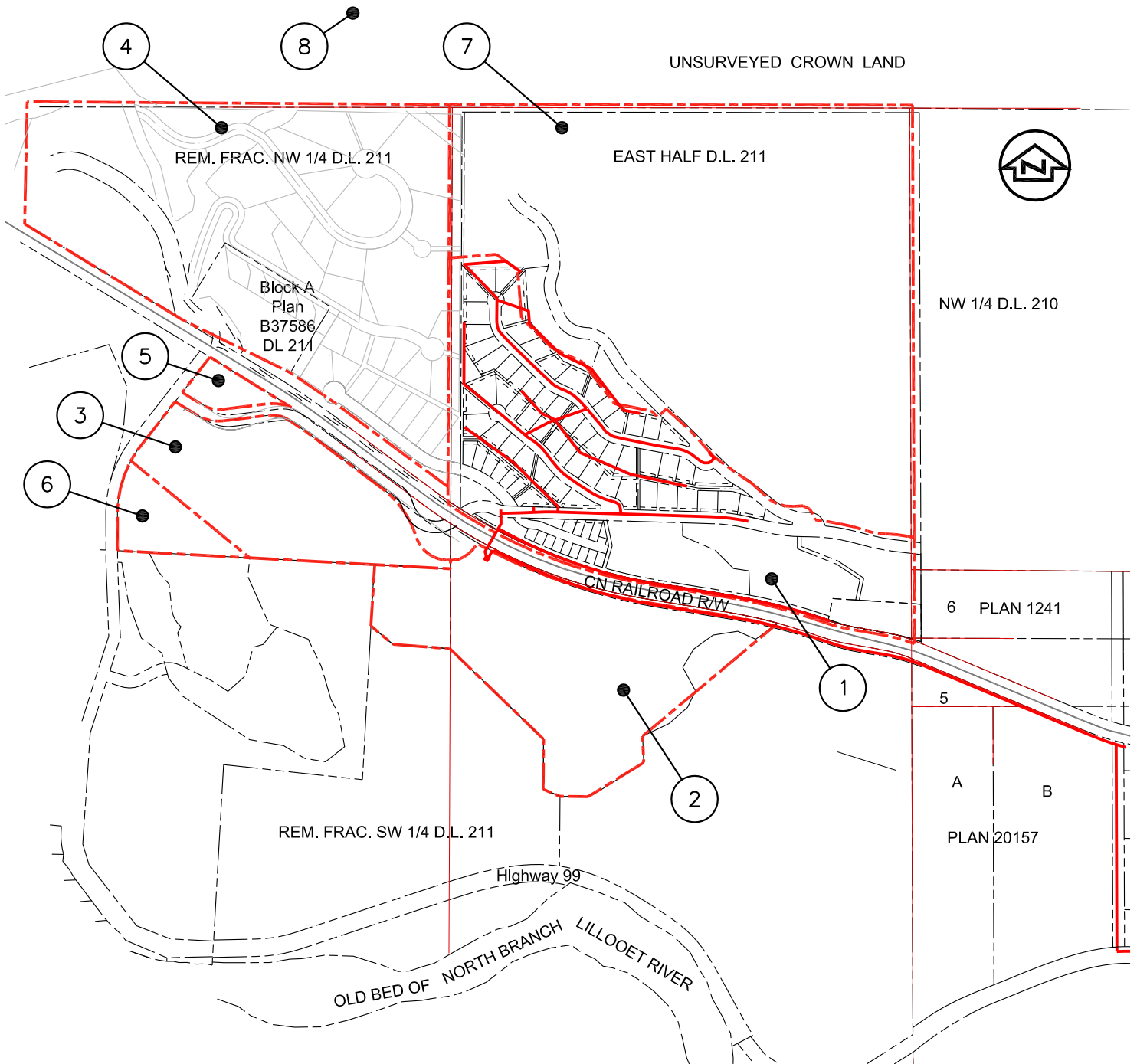
Delcan has been retained to provide engineering services for the development of Phase 1 of the Sunstone Ridge Development (SRD), located in the Village of Pemberton, 3 km east of the Village Centre.

The purpose of this Technical Memorandum is to present the design basis for the sanitary flow assessment, identify the sanitary sewer loadings for the SRD site and other surrounding potential short term development sites.

Development Plan

The SRD site is anticipated to be the first phase of a number of developments in the Sunstone Ridge area. The location of the SRD site and proposed surrounding developments are shown in **Figure 1**. Details on the development plans are as follows:

- | | | |
|----|---|--|
| 1. | Sunstone Ridge Development Site (Phase 1) | 78 single-family units
142 multi-family units |
| 2. | School Site | 1200 student school building
800 student boarding building |
| 3. | Recreation Facility Site | 30,000 ft ² ice arena building
12,000 ft ² swimming pool building |
| 4. | Biro Site | 31 single-family units
77 multi-family units
Resort Hotel |
| 5. | Commercial Site | 100,000 ft ² neighbourhood commercial |
| 6. | 22 SF Site | 22 single-family units |
| 7. | Sunstone Ridge Development Site (Phase 2) | 130 units |



SHORT TERM DEVELOPMENT SITES

- ① SUNSTONE RIDGE SITE
- ② SCHOOL SITE
- ③ RECREATION FACILITY

- ④ BIRO SITE
- ⑤ COMMERCIAL
- ⑥ 22 SF UNITS

LONG TERM DEVELOPMENT SITES

- ⑦ SUNSTONE RIDGE (FUTURE)
- ⑧ LIL'WAT DEVELOPMENT (FUTURE)

Design Parameters

The following documents were referenced to predict the sanitary sewer loads of the SRD site and surrounding short-term development sites:

Village of Pemberton Subdivision and Development Control Bylaw #677, 2011

Squamish Lillooet Regional District (SLRD) Subdivision and Development Servicing Bylaw #741, 2002.

MMCD Design Guideline Manual, 2005.

Sewerage System Standard Practice Manual (SSSPM) Version 2, 2007.

Key parameters used in the assessment are summarized below:

Parameter	Value	Reference
Population per Dwelling	Single Family = 4 people/unit Multi Family = 3 people/unit	SLRD Bylaw #741
Per Capita Demand	Average Dry Weather Flow = 410 litres/capita/day	Pemberton Bylaw #677
Other Demands	Students = 70 L/student/day Boarders = 400 L/boarder/day Arena = 85,000 L/day Swimming Pool = 50 L/m ² Shopping Center = 0.1 L/m ² Restaurant = 125 L/seat	MMCD MMCD MMCD SSSPM MMCD SSSPM
Peaking Factor	PF = $6.75P^{-0.11}$	MMCD Design Guidelines
Infiltration	I = 0.17 L/s/ha	Pemberton Bylaw #677

Sanitary Loading Assessment

Two preliminary servicing designs are being developed for the SRD site, one for servicing only the SRD site, and one for servicing all of the short term potential development sites. This will establish the difference in facilities and costs associated with the SRD site and the neighbouring properties, and may form the basis for cost sharing arrangements such as latecomers' fees. Predicted sanitary sewer loadings from each of the individual sites are summarized in **Table 1**.

Table 1: Summary of Sanitary Loading Predictions

Site	ADWF (L/sec)	I&I (L/sec)	PWWF (L/sec)
1. SRD Site (Phase 1)	3.5	3.9	15.3
2. School Site	4.0	1.1	14.0
3. Recreation Facility	1.6	1.5	7.2
4. Biro Site	2.7	3.4	14.4
5. Commercial Site	0.1	0.1	0.5
6. 22 SF Units	0.4	0.3	2.0
7. SRD Site (Phase 2)	2.5	5.6	13.9

* assumed incl. 50 seat restaurant

Servicing Arrangement 1 - SRD Site Only

The first servicing arrangement is limited to only the SRD site. The arrangement would involve a sanitary pump station and forcemain to pump sewage to the existing 200 mm forcemain at Highway 99 that conveys sewage from the industrial park to the Pemberton Wastewater Treatment Plant. The pump station and forcemain would need to be sized to accommodate 15.3 L/sec.

Servicing Arrangement 2: All Short Term Development Sites

The second servicing arrangement includes the SRD site and the surrounding short term development sites. The overall servicing arrangement would be the same as arrangement 1. The pump station and forcemain for this arrangement would need to be sized to accommodate 59.1 L/sec

Conclusions

To proceed with preliminary design of the sanitary pump station and forcemain required for the SRD development, we require confirmation that there is available capacity in the existing 200mm forcemain servicing the industrial park for an additional sanitary loading scenarios of 15.3 L/sec or 59.1 L/sec. If capacity is available, boundary conditions for the tie-in to the village forcemain for each scenario are required.